

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

ROY COOPER GOVERNOR JAMES H. TROGDON, III
SECRETARY

June 21, 2019

Addendum No. 1

RE: Contract # C204266 WBS # 36030.3.GV4 F. A. # NHPP-026-1(199)6 Buncombe-Henderson Counties (I-4700) I-26 FROM NC-280 (EXIT 40) TO I-40

July 16, 2019 Letting

To Whom It May Concern:

Reference is made to the plans and proposal form furnished to you on this project.

The following revisions have been made to the Roadway plans:

Sheet No.	Revision
New Sheet TMP-	Townson Chair Details I at 1 1 5
002A	Temporary Shoring Data for shoring locations 1 thru 5

The plans furnished to you contained 2 Sheet TMP-002Bs. Void the first Sheet TMP-002B and replace with this New Sheet TMP-002A.

The following revisions have been made to the Structure plans:

Sheet No.	Revision
S4-4	Revised quantities for PERMANENT STEEL CASING FOR 5'-0" DIA. DRILLED PIER under <u>Total Bill of Material</u>
S4-63	Revised quantities for PERMANENT STEEL CASING FOR 5'-0" DIA. DRILLED PIER under Quantities
S4-69	Revised quantities for PERMANENT STEEL CASING FOR 5'-0" DIA. DRILLED PIER under Quantities

Please void the above listed existing Sheets in your proposal and staple the revised Sheets thereto.

The following revisions have been made to the proposal:

Page No.	Revisions
Proposal Cover	Note added that reads "Includes Addendum No. 1 Dated 06-21-2019".
Table of Contents	Deleted INTERMEDIATE CONTRACT TIME NUMBER 1 AND LIQUIDATED DAMAGES. Added INTERMEDIATE CONTRACT TIME NUMBER 1, BONUS CLAUSE AND LIQUIDATED DAMAGES. Added INTERMEDIATE CONTRACT TIME NUMBER 5 AND LIQUIDATED DAMAGES.
G-1 thru G-4	Deleted the project special provision entitled INTERMEDIATE CONTRACT TIME NUMBER 1 AND LIQUIDATED DAMAGES. Added the project special provision entitled INTERMEDIATE CONTRACT TIME NUMBER 1, BONUS CLAUSE AND LIQUIDATED DAMAGES. Revised the project special provision entitled INTERMEDIATE CONTRACT TIME NUMBER 2 AND LIQUIDATED DAMAGES. Revised the project special provision entitled INTERMEDIATE CONTRACT TIME NUMBER 4 AND LIQUIDATED DAMAGES Added the project special provision entitled INTERMEDIATE CONTRACT TIME NUMBER 5 AND LIQUIDATED DAMAGES.
G-6 thru G-7	Revised project special provision entitled CONSTRUCTION MORATORIUM. Revised project special provision entitled DELAY IN RIGHT OF ENTRY.
G-37	Revised the first sentence of the last paragraph on the Page.
G-38 and New Page G-39	Revised the project special provision entitled NOTES TO CONTRACTOR.
R-37 thru R-39	Revised the project special provision entitled DIAMOND GRINDING CONCRETE PAVEMENT.
New Pages BLRI- 174 and BLRI-175	Added the project special provision entitled SEGMENTAL BRIDGE CONTRACTOR AND PERSONNEL.
TC-1	Added Work Zone Traffic Pattern Masking
TC-32 and New Pages TC-33 thru TC-36	Added the project special provision entitled WORK ZONE TRAFFIC PATTERN MASKING.
UbO-1	Revised 2 nd paragraph under General :
ST-1 thru ST-52 and New Pages ST-53 thru ST-57	Added project special provisions entitled Maintenance and Protection of Traffic Beneath Proposed Structure at Station 27+36.45 -Y13- and Application of Bridge Coating.

Please void the above listed existing Pages in your proposal and staple the revised Pages thereto. Staple new Page G-39 after revised Page G-38 in your proposal. Staple new Pages BLRI-174 and BLRI-175 after Page BLRI-173 in your proposal. Staple new Pages TC-33 thru TC-36 after revised Page TC-32 in your proposal. Staple new Pages ST-53 thru ST-57 after revised Page ST-52 in your proposal.

On the item sheets the following pay items have been revised:

<u>Item</u> 265- 4855000000-E- 1205	<u>Description</u> Removal of Pavement Marking Lines (6")	Old Quantity 307,545 LF	New Quantity 61,511 LF
266- 4865000000-E- 1205	Removal of Pavement Marking Lines (12")	16,796 SY	4,113 LF
267-4875000000-N- 1205	Removal of Pavement Marking Symbols & Characters	21 EA	7 EA
444- 8111000000-E- 411	Permanent Steel Casing for 5'-0" Dia. Drilled Pier	344 LF	200.6 LF
488- 4848000000-E- SP	Work Zone Traffic Pattern Masking	New Item	5,979,745 SF
489- 8860000000-N- SP	Application of Bridge Coating	New Item	Lump Sum

The Contractor's bid must include these pay item revisions.

The electronic bidding file has been updated to reflect this revision. Please download the Addendum File and follow the instructions for applying the addendum. Bid Express will not accept your bid unless the addendum has been applied.

The contract will be prepared accordingly.

Sincerely,

--- DocuSigned by:

Ronald E. Davenport, Ir.

----F81B6038A47A442...

Ronald E. Davenport, Jr., PE

State Contract Officer

RED/jjr

Attachments

cc:	Mr. Lamar Sylvester, PE	Mr. Ray Arnold, PE
	Mr. Mark T. Gibbs, PE	Mr. Ken Kennedy, PE
	Mr. Brian Burch, PE	Ms. Jaci Kincaid
	Mr. Ron Hancock, PE	Ms. Lori Strickland
	Mr. Chris Peoples, PE	Mr. Mike Gwyn
	Mr. Jon Weathersbee, PE	Ms. Penny Higgins
	Project File (2)	Mr. Mitchell Dixon

TEMPORARY SHORING DATA

SHORING LOCATION NO. (1)

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR THE ROADWAY CONSTRUCTION FROM STATION -EBL- $895+00\pm$, $2.0'\pm$ RT., TO STATION -EBL $895+00\pm$, $15.0'\pm$ LT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM STATION
-EBL- 895+00±, 2.0'± RT., TO STATION -EBL- 895+00±,
15.0'± LT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS AND GROUNDWATER ELEVATION:

UNIT WEIGHT y = 120 LB/CF FRICTION ANGLE Ø = 30 DEGREES COHESION (c) = 0 LB/SF GROUNDWATER ELEVATION = 2090 FT

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM STATION -EBL-895+00±, 2.0'± RT., TO STATION -EBL-895+00±, 15.0'± LT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL SITE CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

AT THE CONTRACTOR'S OPTION, USE A STANDARD TEMPORARY WALL FOR TEMPORARY SHORING FROM STATION -EBL- 895+00±, 2.0'± RT., TO STATION -EBL- 895+00±, 15.0'± LT. SEE GEOTECHNICAL STANDARD DETAIL NO. 1801.02 FOR STANDARD TEMPORARY WALLS.

SHORING LOCATION NO. (2)

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR THE ROADWAY CONSTRUCTION FROM STATION -EBL- $895+00\pm$, $15.0'\pm$ LT, TO STATION -EBL- $896+50\pm$, $30.5'\pm$ LT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM STATION -EBL- $895\pm00\pm$, $15.0'\pm$ LT, TO STATION -EBL- $896\pm50\pm$, $30.5'\pm$ LT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS AND GROUNDWATER ELEVATION:

UNIT WEIGHT y = 120 LB/CF FRICTION ANGLE Ø = 30 DEGREES COHESION (c) = 0 LB/SF GROUNDWATER ELEVATION = 2095 FT

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM STATION -EBL-895+00±, 15.0'± LT, TO STATION -EBL-896+50±, 30.5'± LT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL SITE CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

AT THE CONTRACTOR'S OPTION, USE A STANDARD TEMPORARY WALL FOR TEMPORARY SHORING FROM STATION -EBL- 895+00±, 15.0'± LT, TO STATION -EBL- 896+50±, 30.5'± LT. SEE GEOTECHNICAL STANDARD DETAIL NO. 1801.02 FOR STANDARD TEMPORARY WALLS.

SHORING LOCATION NO. $\langle 3 \rangle$

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR END BENT CONSTRUCTION FROM STATION -EBL- 911+69±, 47.0'± LT, TO STATION -EBL- 912+30±, 47.0'± LT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM STATION -EBL- 911+69±, 47.0'± LT, TO STATION -EBL- 912+30±, 47.0'± LT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS AND GROUNDWATER ELEVATION:

UNIT WEIGHT y = 120 LB/CF FRICTION ANGLE Ø = 30 DEGREES COHESION (c) = 0 LB/SF GROUNDWATER ELEVATION = 2065 FT

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM STATION -EBL-911+69±, 47.0'± LT, TO STATION -EBL-912+30±, 47.0'± LT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL SITE CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

AT THE CONTRACTOR'S OPTION, USE A STANDARD TEMPORARY WALL FOR TEMPORARY SHORING FROM STATION -EBL- 911+69±, 47.0'± LT, TO STATION -EBL- 912+30±, 47.0'± LT. SEE GEOTECHNICAL STANDARD DETAIL NO. 1801.02 FOR STANDARD TEMPORARY WALLS.

SHORING LOCATION NO. (4)

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR END BENT CONSTRUCTION FROM STATION -EBL- 912+85±, 46.5'± LT, TO STATION -EBL- 913+28±, 46.5'± LT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM STATION -EBL- $912+85\pm$, $46.5'\pm$ LT, TO STATION -EBL- $913+28\pm$, $46.5'\pm$ LT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS AND GROUNDWATER ELEVATION:

UNIT WEIGHT y = 120 LB/CF FRICTION ANGLE Ø = 30 DEGREES COHESION (c) = 0 LB/SF GROUNDWATER ELEVATION = 2050 FT

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM STATION -EBL- 912+85±, 46.5'± LT, TO STATION -EBL- 913+28±, 46.5'± LT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL SITE CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

AT THE CONTRACTOR'S OPTION, USE A STANDARD TEMPORARY WALL FOR TEMPORARY SHORING FROM STATION -EBL- 912+85±, 46.5'± LT, TO STATION -EBL- 913+28±, 46.5'± LT. SEE GEOTECHNICAL STANDARD DETAIL NO. 1801.02 FOR STANDARD TEMPORARY WALLS.

SHORING LOCATION NO. (5)

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR THE ROADWAY CONSTRUCTION FROM STATION -EBL- 892+00±, 10.3'± RT, TO STATION -EBL- 895+00±, 2.0'± RT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM STATION -EBL- 892+00±, 10.3'± RT, TO STATION -EBL- 895+00±, 2.0'± RT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS AND GROUNDWATER ELEVATION:

UNIT WEIGHT y = 120 LB/CF FRICTION ANGLE Ø = 30 DEGREES COHESION (c) = 0 LB/SF GROUNDWATER ELEVATION = 2090 FT

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM STATION -EBL-892+00±, 10.3'± RT, TO STATION -EBL-895+00±, 2.0'± RT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL SITE CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

AT THE CONTRACTOR'S OPTION, USE A STANDARD JEMPORARY WALL FOR TEMPORARY SHORING FROM STATION -EBL-892+00±, 10.3'± RT, TO STATION -EBL-895+00±, 2.0'± RT. SEE GEOTECHNICAL STANDARD DETAIL NO. 1801.02 FOR STANDARD TEMPORARY WALLS.

THE TEMPORARY SHORING NOTES SHOWN ON THIS SHEET WERE PROVIDED THROUGH SEALED DOCUMENTS FROM THE GEOTECHNICAL ENGINEERING UNIT. THE DOCUMENTS WERE SUBMITTED TO THE WZTC SECTION ON FEBRUARY 20, 2019 AND SEALED BY PROFESSIONAL ENGINEER, SHANE CLARK, P.E., LICENSE #29869.

APPROVED State Claft

Transference

5/20/2019

SEAL

SEAL

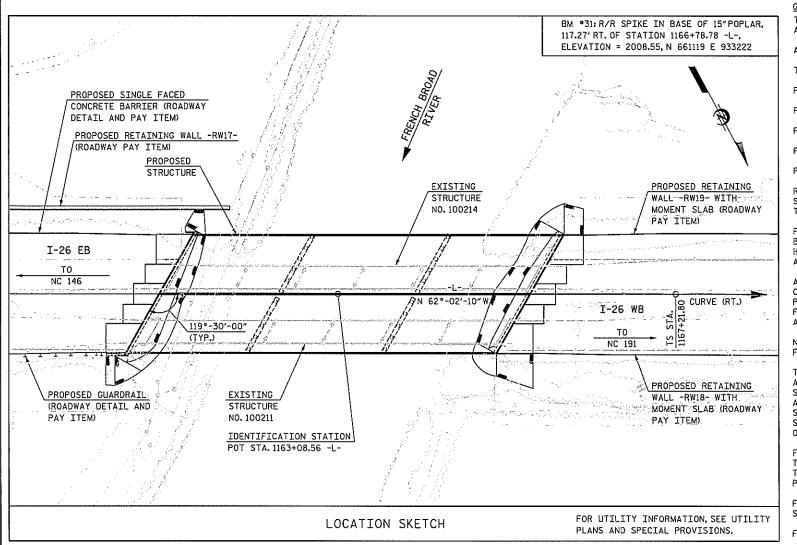
029869



TEMPORARY SHORING NOTES

HNTB MORTH CANOLINA, P.C.
343 E. Six Forks Rond, Suite 200
Raleigh, North Carolina 27609
Rollings Roc C-1554

DOCUMENT NOT CONSIDERED FINAL



GENERAL NOTES:

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS,

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

FOR CAUSEWAY LIMITS AND CONSTRUCTION SEQUENCE, SEE BIOLOGICAL OPINION "I-26 WIDENING FROM US 25 NEAR HENDERSONVILLE TO I-40/I-240 SOUTH OF ASHEVILLE, HENDERSON AND BUNCOMBE COUNTIES, NORTH CAROLINA".

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 1163+08.56 -L-.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANNULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS, FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIAL CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURES AT STATION 1163+08,56 -L-".

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50W AND PAINTED IN ACCORDANCE WITH SYSTEM 5 OR SYSTEM 6 OF THE STRUCTURAL STEEL SHOP COATINGS PROGRAM AND SECTION 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

DIMENSIONS AND ELEVATIONS SHOWN FOR THE EXISTING STRUCTURES ARE FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING STRUCTURES SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE. IF FIELD CONDITIONS VARY FROM THE PLANS, MODIFICATIONS MAY BE MADE AS NECESSARY AS DIRECTED BY THE ENGINEER.

REMOVAL OF THE EXISTING BRIDGES SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER, THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THEC-18 EVALUATING SCOUR AT BRIDGES".

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE SAMPLE BARS SHOULD COME FROM STEEL ACTUALLY USED IN THE PROJECT AND THE SAMPLE BARS SHOULD BE REPLACED BY SPLICED BARS AS SPECIFIED IN THE SAMPLE BAR REPLACEMENT CHART, PAYMENT FOR THE SAMPLE BARS AND REPLACEMENT REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

EXISTING STRUCTURE NO. 100214 WITH ONE END SPAN LENGTH OF 87'-0". INTERIOR SPAN LENGTHS OF 73'-0", 73'-0", 73'-0", AND 73'-0", AND ONE END SPAN LENGTH OF 74'-6" WITH REINFORCED CONCRETE DECK SUPPORTED BY 4 LINES OF 36"STEEL I-BEAMS AT 8'-0"CTS. AND 28'-0"CLEAR ROADWAY WIDTH ON REINFORCED CONCRETE END BENT CAPS ON H-PILES AND REINFORCED CONCRETE BENT POST AND BEAM ON SPREAD FOOTINGS LOCATED ±21' DOWNSTREAM OF THE PROPOSED STRUCTURE SHALL BE REMOVED. EXISTING STRUCTURE NO. 100211 WITH ONE END SPAN LENGTH OF 74'-6", INTERIOR SPAN LENGTHS OF 73'-0", 73'-0", AND 73'-0", AND ONE END SPAN LENGTH OF 74'-6"WITH REINFORCED CONCRETE DECK SUPPORTED BY 4 LINES OF 36"STEEL I-BEAMS AT 8'-0"CTS, AND 28'-0"CLEAR ROADWAY WIDTH ON REINFORCED CONCRETE END BENT CAPS ON H-PILES AND REINFORCED CONCRETE BENT POST AND BEAM ON SPREAD FOOTINGS LOCATED ±46' UPSTREAM OF THE PROPOSED STRUCTURE SHALL ALSO BE REMOVED. BOTH EXISTING BRIDGES ARE PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGES DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

FOR SECURING OF VESSELS, SEE SPECIAL PROVISIONS.

RIVER TRAFFIC SHALL BE MAINTAINED DURING BRIDGE CONSTRUCTION, FOR REQUIREMENTS, SEE WORK ZONE TRAFFIC CONTROL FOR SPECIAL PROVISIONS.

	TOTAL BILL OF MATERIAL														
	CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION STA.1163+08.56 -L-	REMOVAL OF EXISTING STRUCTURES AT STA.1163+08.56 -L-	ASBESTOS ASSESSMENT	5'-0"DIA. DRILLED PIERS IN SOIL	5'-0"DIA. DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 5'-O"DIA. DRILLED PIER	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION AT STA.1163+08.56 -L-	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE				
	LUMP SUM	LUMP SUM	LUMP SUM	L.F.	L.F.	L.F.	EACH	LUMP SUM	SO. FEET	SO. FEET	CU. YARDS				
SUPERSTRUCTURE	***************************************	***************************************							68,878	72,841					
END BENT 1											201.5				
BENT 1		***************************************		81.3	147.0	103.3					280.9				
BENT 2	<u> </u>			75.3	153.0	97.3					290.4				
END BENT 2											175.2				
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	156.6	300.0	200.6	4	LUMP SUM	68,878	72,841	948.0				

	TOTAL BILL OF MATERIAL														
	BRIDGE APPR. SLABS, STATION STA.1163+08.56 -L-		SPIRAL COLUMN REINFORCING STEEL		PILE DRIVING EQUITMENT SETUP FOR HP 14 x 73 STEEL PILES	HP 14 X 75 1		CONCRETE BARRIER RAIL	CONCRETE MEDIAN BARRIER	RIP-RAP CLASS II (2'-O"THICK)	GEOTEXTILE FOR DRAINAGE	DISC BEARINGS	EXPANSION JOINT SEALS		
	LUMP SUM	LBS.	LBS.	LUMP SUM	EACH	NO.	L.F.	L.F.	L.F.	TON	SQ. YD.	LUMP SUM	LUMP SUM		
SUPERSTRUCTURE	LUMP SUM			LUMP SUM				1,050.20	526.00			LUMP SUM	LUMP SUM		
END BENT 1		24,109			27	27	540			914.8	1,016.4				
BENT 1		131,930	14,212												
BENT 2		134,323	14,641									*******			
END BENT 2		21,739			27	27	810			1,328.6	1,476.3				
TOTAL	LUMP SUM	312,101	28,853	LUMP SUM	54	54	1,350	1,050.20	526.00	2,243.4	2,492.7	LUMP SUM	LUMP SUM		

NOTE:

SAMPLE BAR.

SAMPLE BAR REPLACEMENT LENGTHS BASED ON 30" (SAMPLE LENGTH) PLUS TWO SPLICE LENGTHS AND	PROJECT N	10.	I-4700B					
fy = 60ksi.BAR LENGTHS IN THIS TABLE ARE A	BU	NCOM	IBE	_coi	YTNL			
GUIDE THE ENGINEER SHALL APPROVE FINAL LENGTHS BASED ON THE	STATION:	<u> POT</u>	1163+08	.56	<u>-L-</u>			
TYPE AND LOCATION OF								

SHEET 4 OF 4

DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

LOCATION SKETCH, GENERAL NOTES. AND TOTAL BILL OF MATERIAL

HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Roleigh, N.C. 27609 HNTB

SAMPLE BAR REPLACEMENT

SIZE LENGTH

6'-2"

7'-4"

8'-6"

9'-8"

10'-10"

12'-0"

13'-2"

14'-6"

15'-10"

#3

#5

***6** #7

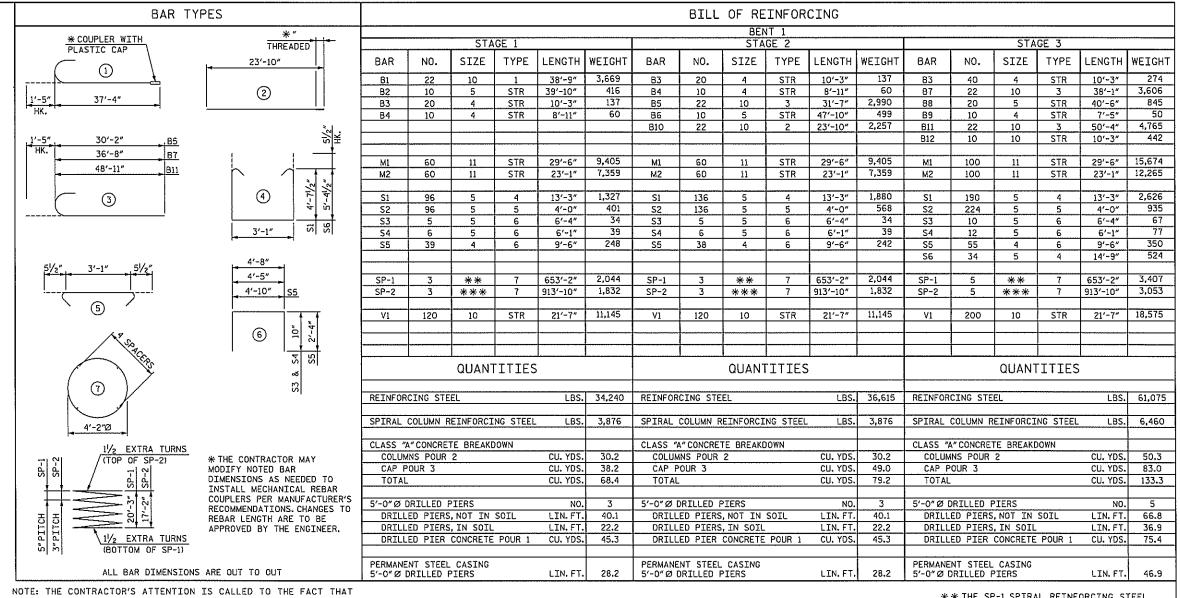
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*10

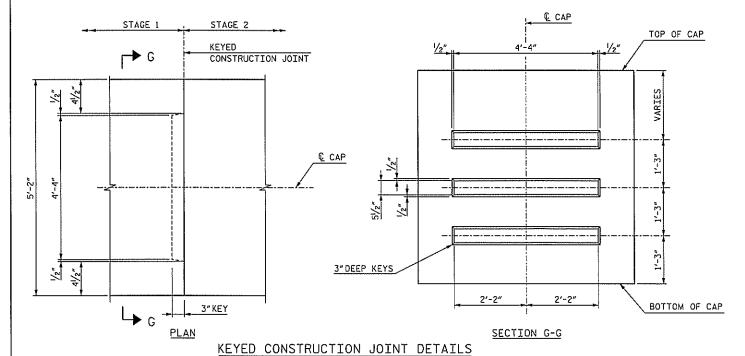
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REVISIONS NO. BY DATE NO. BY DATE S4-4 DRAWN BY B. VAUGHN
CHECKED BY R. RAPP
DESIGN ENGINEER OF RECORD R. RAPP 3

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



THE LONGINTUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH. "M2" BARS SHALL BE FIELD CUT AS NECESSARY TO MAINTAIN 6"CLEARANCE FROM THE CONSTRUCTION JOINT BETWEEN THE COLUMN AND DRILLED PIER.



** THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR.

*** THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.

I-4700B PROJECT NO. BUNCOMBE COUNTY STATION: POT 1163+08.56 -L-

SHEET 6 OF 6

DEPARTMENT OF TRANSPORTATION

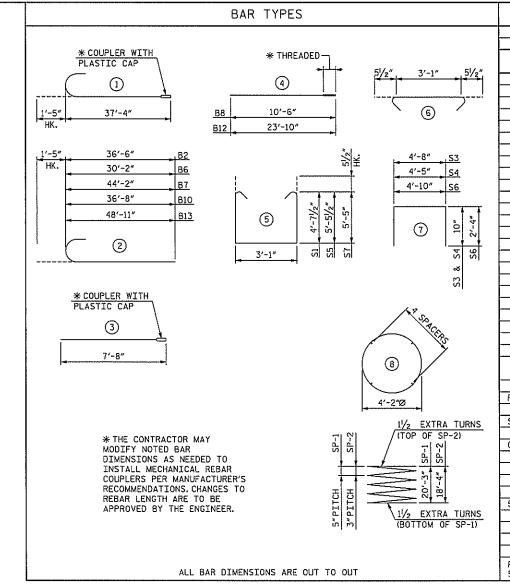
SUBSTRUCTURE

BENT 1

HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27608 HNTB DRAWN BY B. VAUGHN
CHECKED BY R. RAPP
DESIGN ENGINEER OF RECORD R. RAPP

SHEET NO. REVISIONS S4-63 NO. BY DATE NO. BY DATE SHEETS 89 DWG. NO. 63 3

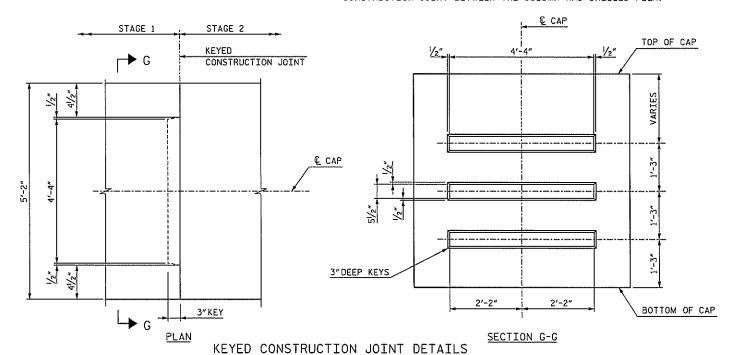
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



NO.		BENT 2 STAGE 1 STAGE 2 STAGE 3															
	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
12	10	1	38'-9"	2,001	85	20	4	STR	10'-3"	137	B5	40	4	STR	10'-3"	274	
10	10	2	37'-11"	1,632	B6	12	10	2	31'-7"	1,631	B10	22	10	2	38'-1"	3,606	
10	10	3	7′-8″	330	B7	10	10	2	45'-7"	1,962	811	20	5	STR	40'-6"	845	
	_									l						4,765	
											814	10	10	STR	10'-3"	442	
10	4	STR	8'-2"	55	B12	12	10	4	23'-10"	1,231							
	11	CTD	201-6"	9 405			11	CTD	20/_6#	9 405	1.11	100	11	СТР	201_6#	15,674	
							1									12,265	
- 00	- 11	3111	23 1	1,005	IVIZ	00		2111			IYFC	100		3111			
82	5	5	13'-3"	1,134	S1	94	5	5	13′-3″	1,300	S1	190	5	5	13'-3"	2,626	
96	5	6	4'-0"	401	52	136	5	6	4'-0"	568	S2	224	5	6	4'-0"	935	
5	5	7	6'-4"	34	S3	5	5	7	6'-4"	34	\$3	10	5	7	6'-4"	67	
6	5	7	6'-1"		54	6	5	7	6′-1″		S4	12	5	7	6'-1"	77	
			14'-11"													292	
39	4	7	9′~6"	248	\$6	27	4	7	9'-6"	172	S7	34	5	5	14'-10"	527	
7	ale ale	0	CE31 0//	2 044	CD 1	7	**		CE31 0#	2 044	CO. 1		***		CE31. 3#	3,407	
																3,248	
J	***	0	312 -2	*,5,5	31-2	,	***		216.6	1,0 15	31 4	,	***	-	312 2	3,2.10	
120	10	STR	22'-9"	11,748	V1	120	10	STR	22'-9"	11,748	V1	200	10	STR	22'-9"	19,580	
				······································													
	CHANT	TTTFS					ΠΙΔΝΤ	TTTES					ΩΠΔΝΤ	TTTFS			
	GOAITI	11110					40/111	* · * * * * *					QUAIT!	<u> </u>			
THE STE	FI		LDC	75 167	DETAILOR	CTNC CTC	F1		100	77 101	DETNEAD	CTNC CTC	<u>~</u> 1		100	61,975	
THE SIE	<u>LL</u>		LD3.	33,137	KETIALOKI	TING DIE	C.L.		LDS.	31,131	ALLIN ONCING STEEL						
OLLIMN R	FINEORCI	NG STEEL	LBS.	3,993	SPTRAL (COLUMN R	ETNEORCE	NG STEEL	LBS.	3,993	SPTRAL (COLUMN R	FINEORCI	NG STEEL	LBS.	6,655	
				0,000	0. 2					-,							
CONCRET	E BREAKD	OWN			CLASS "A	"CONCRE	TE BREAKE	OWN			CLASS "A	"CONCRE"	TE BREAKD	OWN			
S POUR	2		CU. YDS.	32.3	COLUM	NS POUR	2		CU. YDS.	32.3	COLUM	NS POUR	2		CU. YDS.	53.8	
บR 3			CU. YDS.	40.3	CAP P	OUR 3			cu. yos.	50.1		OUR 3			CU. YDS.	81.6	
			CU. YDS.	72.6	TOTAL				CU. YDS.	82.4	TOTAL				CU. YDS.	135.4	
													***	007		5	
		20TF						201r				~~~~	····	201F	····	69.6	
		DOLLD 1					 	DOLED 1						POUD 1		34.3 75.4	
U PIEK	CONCRETE	rouk I	CU. 105.	45.3	DRIFT	ED LIEK	CONCRETE	FUUR I	CU. 105.	45.5	DKTEF	בה גדבע	CONCRETE	LOOK I	CO. 105.	15.4	
T STEEL	CASTNO				DEDMANE	NT STEE!	CASTNO			ļ	DEBMANE	NT STEEL	CASTNO				
			LIN.FT.	26.5					LIN. FT.	26.5					LIN.FT.	44.3	
1 C C C	10 10 20 10 60 60 82 96 5 6 14 39 3 3 3 120 ENG STE DLUMN R CONCRETS POUR JR 3	10 10 10 5 20 4 10 4 10 4 10 4 10 11 60 11 82 5 96 5 5 5 6 5 14 5 39 4 3 *** 120 10 QUANT CONCRETE BREAKE S POUR 2 JR 3 ILLED PIERS D PIERS, NOT IN D PIERS, IN SOIL	10 10 3 10 5 STR 20 4 STR 20 4 STR 10 4 STR 60 11 STR 60 11 STR 60 11 STR 60 5 5 6 5 5 7 6 5 5 7 6 5 5 7 6 5 5 7 6 5 5 7 6 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 6 9 7 7 7 9 7 7 7 9 7 9 7 7 7 9 7 9 7 9 7	10 10 3 7'-8" 10 5 STR 39'-10" 20 4 STR 10'-3" 10 4 STR 8'-2" 60 11 STR 29'-6" 60 11 STR 23'-1" 82 5 5 13'-3" 96 5 6 4'-0" 5 5 7 6'-4" 39 4 7 9'-6" 3 *** 8 653'-2" 120 10 STR 22'-9" CUANTITIES CONCRETE BREAKDOWN SPOUR 2 CU. YDS. SPOUR 2 CU. YDS. CU. YDS. CU. YDS. CU. YDS. CU. YDS. CO. PIERS, NOT IN SOIL LIN. FT. CO PIERS, IN SOIL LIN. FT. CO PIERS, IN SOIL LIN. FT. CO PIER CONCRETE POUR 1 CU. YDS. T STEEL CASING	10 10 3 7'-8" 330 10 5 STR 39'-10" 416 20 4 STR 10'-3" 137 10 4 STR 8'-2" 55 60 11 STR 29'-6" 9,405 60 11 STR 23'-1" 7,359 60 5 6 4'-0" 401 5 5 7 6'-4" 34 6 5 7 6'-4" 39 14 5 5 14'-11" 218 39 4 7 9'-6" 248 3 *** 8 653'-2" 2,044 3 *** 8 972'-2" 1,949 120 10 STR 22'-9" 11,748 CUANTITIES CONCRETE BREAKDOWN S POUR 2 CU. YDS. 32.3 JR 3 CU. YDS. 40.3 CU. YDS. 72.6 ILLED PIERS NO. 3 PIERS, NOT IN SOIL LIN. FT. 41.7 O PIERS, IN SOIL LIN. FT. 41.7 O PIERS, IN SOIL LIN. FT. 20.5 O PIER CONCRETE POUR 1 CU. YDS. 45.3 I STEEL CASING	10 10 3 7'-8" 330 B7 10 5 STR 39'-10" 416 B8 20 4 STR 10'-3" 137 B9 10 4 STR 8'-2" 55 B12 60 11 STR 29'-6" 9,405 M1 60 11 STR 23'-1" 7,359 M2 82 5 5 13'-3" 1,134 S1 96 5 6 4'-0" 401 S2 5 5 7 6'-4" 339 S4 14 5 5 14'-11" 218 S5 39 4 7 9'-6" 248 S6 3 *** 8 972'-2" 1,949 SP-2 120 10 STR 22'-9" 11,748 V1 QUANTITIES ING STEEL LBS. 3,993 SPIRAL 6 CONCRETE BREAKDOWN SPOUR 2 CU. YDS. 32.3 COLUMN SPOUR 2 CU. YDS. 32.3 COLUMN SPOUR 2 CU. YDS. 72.6 TOTAL SPOUR SPOUR 2 CU. YDS. 72.6 TOTAL SPOUR SPOU	10 10 5 STR 39'-10" 416 B8 10 20 4 STR 10'-3" 137 B9 10 10 4 STR 8'-2" 55 B12 12 60 11 STR 29'-6" 9,405 M1 60 60 11 STR 23'-1" 7,359 M2 60 82 5 5 13'-3" 1,134 S1 94 96 5 6 4'-0" 34 S3 5 6 5 7 6'-4" 34 S3 5 6 5 7 6'-1" 39 S4 6 14 5 5 14'-11" 218 S5 42 39 4 7 9'-6" 248 S6 27 3 *** 8 653'-2" 2,044 SP-1 3 3 *** 8 972'-2" 1,949 SP-2 3 120 10 STR 22'-9" 11,748 V1 120 QUANTITIES ENG STEEL LBS. 3,993 SPIRAL COLUMN R CONCRETE BREAKDOWN CLASS "A" CONCRETE BREAKDOWN SPOUR 2 UNITS SPOUR 2 CU. YDS. 32.3 COLUMNS POUR 3 CONCRETE BREAKDOWN CLASS "A" CONCRETE BREAKDOWN SPOUR 2 UNITS SPOUR 2 CU. YDS. 32.3 COLUMNS POUR 3 CU. YDS. 40.3 CAP POUR 3 CU. YDS. 72.6 TOTAL ILLED PIERS NO. 3 5'-0" Ø DRILLED FERS OPIERS, NOT IN SOIL LIN. FT. 41.7 DRILLED PIERS OPIERS, IN SOIL LIN. FT. 41.7 DRILLED PIERS OPIERS CONCRETE POUR 1 CU. YDS. 45.3 DRILLED PIERS TO STEEL CASING PERMANENT STEEL	10	10	10 10 3 7'-8" 330 B7 10 10 2 45'-7" 10 5 STR 39"-10" 416 B8 10 10 4 10'-6" 20 4 STR 10'-3" 137 B9 10 5 STR 47'-10" 10 4 STR 8'-2" 55 B12 12 10 4 23'-10" 60 11 STR 29'-6" 9,405 M1 60 11 STR 29'-6" 60 11 STR 23'-1" 7,359 M2 60 11 STR 23'-1" 82 5 5 13'-3" 1,134 S1 94 5 5 13'-3" 96 5 6 4'-0" 401 S2 136 5 6 4'-0" 5 5 7 6'-4" 34 S3 5 5 7 6'-4" 14 5 5 14'-11" 218 S5 42 5 5 14'-11" 39 4 7 9'-6" 248 S6 27 4 7 9'-6" 3 *** 8 653'-2" 2,044 SP-1 3 *** 8 653'-2" 120 10 STR 22'-9" 11,748 V1 120 10 STR 22'-9" 120 10 STR 22'-9" 11,748 V1 120 I0 STR 22'-9" 120 10 STR 22'-9" 11,748 V1 1	10 10 3 7'-8" 330 B7 10 10 2 45'-7" 1,962 10 5 STR 39'-10" 416 B8 10 10 4 10'-6" 452 20 4 STR 10'-3" 137 B9 10 5 STR 47'-10" 499 10 4 STR 8'-2" 55 B12 12 10 4 23'-10" 1,231 60 11 STR 29'-6" 9,405 M1 60 11 STR 29'-6" 9,405 60 11 STR 23'-1" 7,359 M2 60 11 STR 23'-1" 7,359 82 5 5 13'-3" 1,134 S1 94 5 5 13'-3" 1,300 96 5 6 4'-0" 401 52 136 5 6 4'-0" 568 5 5 7 6'-4" 34 S3 5 5 7 6'-4" 34 6 5 7 6'-1" 39 S4 6 5 7 6'-1" 39 14 5 5 14'-11" 218 S5 42 5 5 14'-11" 654 39 4 7 9'-6" 248 S6 27 4 7 9'-6" 172 3 *** 8 653'-2" 2,044 SP-1 3 *** 8 653'-2" 2,044 3 *** 8 653'-2" 2,044 SP-1 3 *** 8 972'-2" 1,949 120 10 STR 22'-9" 11,748 V1 120 10 STR 22'-9" 11,748 OUANTITIES QUANTITIES QUANTITIES	10	10	10	10	10	

BILL OF REINFORCING

NOTE: THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGINTUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH. "M2" BARS SHALL BE FIELD CUT AS NECESSARY TO MAINTAIN 6" CLEARANCE FROM THE CONSTRUCTION JOINT BETWEEN THE COLUMN AND DRILLED PIER.



** THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D31 COLD DRAWN WIRE OR *5 PLAIN OR DEFORMED BAR.

*** THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.

 PROJECT NO.
 I-4700B

 BUNCOMBE
 COUNTY

 STATION:
 POT 1163+08.56 -L

SHEET 6 OF 6

DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

BENT 2

HNTB NORTH CAROLINA, P.C.

NC License No. C-1554
343 E. Six Forke Rd., Suite 200, Roleigh, N.C. 27609

DRAWN BY B. VAUGHN
CHECKED BY R. RAPP.
DESIGN ENGINEER OF RECORD B. RAPP.
DATE 2/19
DESIGN ENGINEER OF RECORD B. RAPP.
DATE 2/19

| REVISIONS | SHEET NO. | SHEET NO. | S4-69 | S4-69 | SHEET NO. | S4-69 | S4-69 | SHEET NO. | S4-69 | S4-69 | SHEET NO. | S4-69 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED